

Table 5.21a—Uplift Loads (SI)

Uplift Load Case	Net Uplift Formula, $U$ (N)	Allowable Anchor Bolt or Anchor Strap Stress (MPa)	Allowable Shell Stress at Anchor Attachment (MPa)	
Design Pressure	$[P_i \times D^2 \times 785] - W_1$	$^{5/12} \times F_y$	$^{2/3} F_{ty}$	14
Test Pressure	$[P_t \times D^2 \times 785] - W_3$	$^{5/9} \times F_y$	$^{5/6} F_{ty}$	15
Wind Load	$P_{WR} \times D^2 \times 785 + [4 \times M_{WS}/D] - W_2$	$0.8 \times F_y$	$^{5/6} F_{ty}$	15
Seismic Load	$[4 \times M_{rw}/D] - W_2 (1 - 0.4A_v)$	$0.8 \times F_y$	$^{5/6} F_{ty}$	15
Design Pressure <sup>a</sup> + Wind	$[F_p (P_i + P_{WR}) \times D^2 \times 785] + [4 M_{WS}/D] - W_1$	$^{5/9} \times F_y$	$^{5/6} F_{ty}$	15
Design Pressure <sup>a</sup> + Seismic	$[F_p P_i \times D^2 \times 785] + [4 M_{rw}/D] - W_1 (1 - 0.4A_v)$	$0.8 \times F_y$	$^{5/6} F_{ty}$	15
Frangibility Pressure <sup>b</sup>	$[3 \times P_f \times D^2 \times 785] - W_3$	$F_y$	$F_{ty}$	
where				
$A_v$	is the vertical earthquake acceleration coefficient, in % g;			
$D$	is the tank diameter, in meters;			
$F_p$	is the pressure combination factor;			
$F_{ty}$	is the minimum yield strength of the bottom shell course, in MPa;			
$F_y$	is the minimum yield strength of the anchor bolt or strap; bolts are limited to specified material minimum yield strength or 380 MPa, whichever is less, in MPa; anchor strap material minimum yield strength shall not exceed the minimum yield strength of the shell;			
$H$	is the tank height, in meters;			
$M_{WS}$	equals $P_{WS} \times D \times H^2/2$ , in N-m;			
$M_{rw}$	is the seismic moment, in N-m (see Annex E);			
$P_i$	is the design internal pressure, in kPa (see Annex F);			
$P_f$	is the failure pressure, in kPa (see Annex F);			
$P_t$	is the test pressure, in kPa (see Annex F);			
$P_{WR}$	is the wind uplift pressure on roof, in kPa;			
$P_{WS}$	is the wind pressure on shell, in N/m <sup>2</sup> ;			
$W_1$	is the corroded weight of the roof plates plus the corroded weight of the shell and any other corroded permanent attachments acting on the shell, in N;			
$W_2$	is the corroded weight of the shell and any corroded permanent attachments acting on the shell including the portion of the roof plates and framing acting on the shell, in N;			
$W_3$	is the nominal weight of the roof plates plus the nominal weight of the shell and any other permanent attachments acting on the shell, in N.			
<sup>a</sup> Refer to 5.2.2 concerning the pressure combination factor applied to the design pressure.				
<sup>b</sup> Frangibility pressure applies only to tanks designed to 5.10.2.6 d.				

Table 5.21b—Uplift Loads (USC)

	Uplift Load Case	Net Uplift Formula, U (lbf)	Allowable Anchor Bolt or Anchor Strap Stress (lbf/in. <sup>2</sup> )	Allowable Shell Stress at Anchor Attachment (lbf/in. <sup>2</sup> )
15	Design Pressure	$[P_i \times D^2 \times 4.08] - W_1$	$5/12 \times F_y$	$2/3 F_{ty}$
15	Test Pressure	$[P_t \times D^2 \times 4.08] - W_3$	$5/9 \times F_y$	$5/6 F_{ty}$
15	Wind Load	$P_{WR} \times D^2 \times 4.08 + [4 \times M_{WS}/D] - W_2$	$0.8 \times F_y$	$5/6 F_{ty}$
	Seismic Load	$[4 \times M_{rw}/D] - W_2 (1 - 0.4A_v)$	$0.8 \times F_y$	$5/6 F_{ty}$
15	Design Pressure <sup>a</sup> + Wind	$[(F_p (P_i + P_{WR}) \times D^2 \times 4.08) + [4 M_{WS}/D] - W_1$	$5/9 \times F_y$	$5/6 F_{ty}$
15	Design Pressure <sup>a</sup> + Seismic	$[F_p P_i \times D^2 \times 4.08] + [4 M_{rw}/D] - W_1 (1 - 0.4A_v)$	$0.8 \times F_y$	$5/6 F_{ty}$
15	Frangibility Pressure <sup>b</sup>	$[3 \times P_f \times D^2 \times 4.08] - W_3$	$F_y$	$F_{ty}$
<p>where</p> <p><math>A_v</math> is the vertical earthquake acceleration coefficient, in % g;</p> <p><math>D</math> is the tank diameter, in feet;</p> <p><math>F_p</math> is the pressure combination factor;</p> <p><math>F_{ty}</math> is the minimum yield strength of the bottom shell course, in psi;</p> <p><math>F_y</math> is the minimum yield strength of the anchor bolt or strap; bolts are limited to specified material minimum yield strength or 55,000 psi, whichever is less, in psi; anchor strap material minimum yield strength shall not exceed the minimum yield strength of the shell;</p> <p><math>H</math> is the tank height, in feet;</p> <p><math>M_{WS}</math> equals <math>P_{WS} \times D \times H^2/2</math>, in ft-lbs;</p> <p><math>M_{rw}</math> is the seismic moment, in ft-lbs (see Annex E);</p> <p><math>P_i</math> is the design internal pressure, in inches of water column (see Annex F);</p> <p><math>P_f</math> is the failure pressure, in inches of water column (see Annex F);</p> <p><math>P_t</math> is the test pressure, in inches of water column (see Annex F);</p> <p><math>P_{WR}</math> is the wind uplift pressure on roof, in inches of water column;</p> <p><math>P_{WS}</math> is the wind pressure on shell, in lbs/ft<sup>2</sup>;</p> <p><math>W_1</math> is the corroded weight of the roof plates plus the corroded weight of the shell and any other corroded permanent attachments acting on the shell, in lbf;</p> <p><math>W_2</math> is the corroded weight of the shell and any corroded permanent attachments acting on the shell including the portion of the roof plates and framing acting on the shell, in lbf;</p> <p><math>W_3</math> is the nominal weight of the roof plates plus the nominal weight of the shell and any other permanent attachments acting on the shell, in lbf.</p> <p><sup>a</sup> Refer to 5.2.2 concerning the pressure combination factor applied to the design pressure.</p> <p><sup>b</sup> Frangibility pressure applies only to tanks designed to 5.10.2.6 d.</p>				